

Rock Flow Simulation by High-Order Quasi-Characteristics Scheme

Mikhail P. Levin

*Ivannikov Institute of System Programming of Russian Academy of
Sciences,*

Moscow, Russia

Mikhail_Levin@hotmail.com; mlevin@ispras.ru

Introduction

- ▶ A pure second-order scheme of quasi-characteristics based on a pyramidal stencil is applied to the numerical modelling of non-stationary two-phase flows through porous media with the essentially heterogeneous properties.
- ▶ In contrast to well-known other high-resolution schemes with monotone properties, this scheme preserves a second-order approximation in regions, where discontinuities of solutions arise, as well as monotone properties of numerical solutions in those regions despite of well-known Godunov theorem.
- ▶ It is possible because the scheme under consideration is defined on a non-fixed stencil and is a combination of two high-order approximation scheme solutions with different dispersion properties.
- ▶ A special criterion according to which, one or another admissible solution is chosen, plays a key role in this scheme. A simple criterion with local character suitable for parallel computations is proposed.
- ▶ Some numerical results showing the efficiency of present approach in computations of two-phase flows through porous media with strongly discontinuous penetration coefficients are presented.

GOVERNING EQUATIONS

► Governing equations

$$m\left(\frac{\partial s}{\partial t}\right) - \frac{\partial}{\partial x}\left(\frac{kk_w}{\mu_w} \frac{\partial p}{\partial x}\right) - \frac{\partial}{\partial y}\left(\frac{kk_w}{\mu_w} \frac{\partial p}{\partial y}\right) = 0, \quad (1)$$

$$\frac{\partial}{\partial x}\left[k\left(\frac{k_w}{\mu_w} + \frac{k_o}{\mu_o}\right) \frac{\partial p}{\partial x}\right] + \frac{\partial}{\partial y}\left[k\left(\frac{k_w}{\mu_w} + \frac{k_o}{\mu_o}\right) \frac{\partial p}{\partial y}\right] = 0. \quad (2)$$

► Transformed transport equation

$$\begin{aligned} \frac{\partial s}{\partial t} - \left(\frac{k}{m\mu_w} \frac{\partial p}{\partial x} \frac{dk_w}{ds}\right) \frac{\partial s}{\partial x} - \left(\frac{k}{m\mu_w} \frac{\partial p}{\partial y} \frac{dk_w}{ds}\right) \frac{\partial s}{\partial y} = \\ = \frac{k_w}{m} \left[\frac{\partial}{\partial x} \left(\frac{k}{\mu_w} \frac{\partial p}{\partial x}\right) + \frac{\partial}{\partial y} \left(\frac{k}{\mu_w} \frac{\partial p}{\partial y}\right) \right]. \end{aligned} \quad (3)$$

INITIAL AND BOUNDARY CONDITIONS

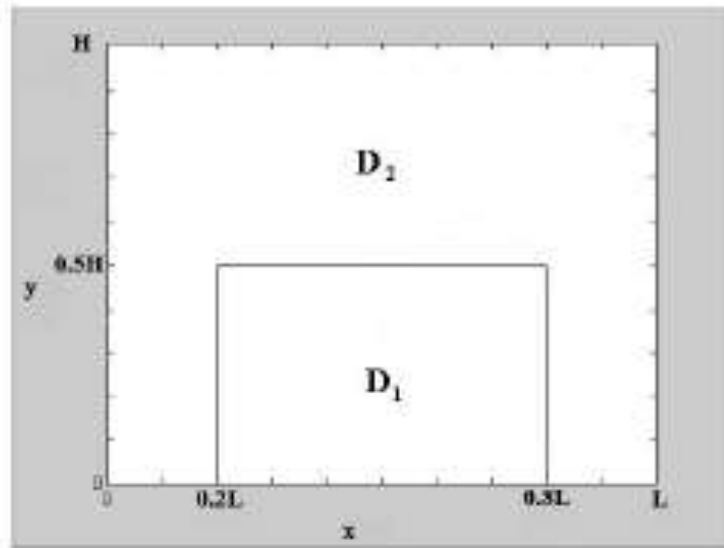


Fig.1. Flow region.

The absolute permeability factor k in each subregion is a constant function, therefore in all region we have

$$k = \begin{cases} k_{D_1}, & \text{if } (x, y) \in D_1, \\ k_{D_2}, & \text{if } (x, y) \in D_2. \end{cases} \quad (4)$$

Initial conditions for the transport equation (3) are

$$s(x, y, 0) = \begin{cases} 0.2, & \text{if } 0 \leq x < L, 0 \leq y \leq H, \\ 1.0, & \text{if } x = L, 0 \leq y \leq H \end{cases} \quad (5)$$

and the boundary conditions are

$$\begin{aligned} \frac{\partial s}{\partial y} &= 0, & \text{if } t > 0, y = 0, H, 0 \leq x \leq L; \\ s(L, y, t) &= 1.0, & \text{if } 0 \leq y \leq H, t > 0. \end{aligned} \quad (6)$$

For the pressure equation (2) of the elliptic type we set a mixed Neumann and Dirichlet boundary conditions as follows

$$\begin{aligned} \frac{\partial p}{\partial y} &= 0, & \text{if } 0 < x < L, y = 0, H; \\ p &= P_0, & \text{if } x = 0, 0 \leq y \leq H \\ \frac{\partial p}{\partial x} &= \frac{Q_0 \mu_w}{H k k_w}, & \text{if } x = L, 0 \leq y \leq H. \end{aligned} \quad (7)$$

P_0 and Q_0 are known parameters here. The relative permeability factors of the water k_w and oil k_o are chosen as follows

$$k_w(s) = \begin{cases} 0, & \text{if } s \leq 0.1; \\ \left(\frac{s-0.1}{0.7}\right)^3, & \text{if } 0.1 < s \leq 0.8; \\ 1, & \text{if } s > 0.8; \end{cases} \quad (8)$$

$$k_o(s) = \begin{cases} 1, & \text{if } s \leq 0.1; \\ \left(\frac{0.8-s}{0.7}\right)^3, & \text{if } 0.1 < s \leq 0.8; \\ 0, & \text{if } s > 0.8. \end{cases} \quad (9)$$

Numerical Scheme

- ▶ Transport equation in generalized characteristic form

$$\frac{\partial u}{\partial t} + b_1 \frac{\partial u}{\partial x} + b_2 \frac{\partial u}{\partial y} = b_3,$$

- ▶ Transport equation in expanded characteristic form

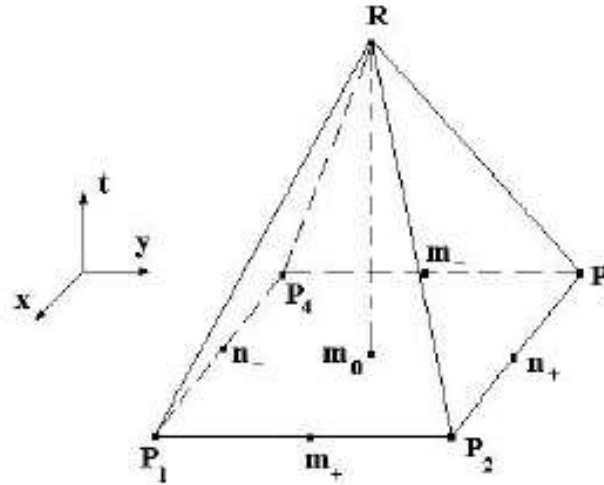
$$\left(\frac{du}{dt}\right)_l + [b_1 - \left(\frac{dx}{dt}\right)_l] \frac{\partial u}{\partial x} + [b_2 - \left(\frac{dy}{dt}\right)_l] \frac{\partial u}{\partial y} = b_3$$

- ▶ Approximation of outward derivatives at the middle layer ($t_0 + \tau/2$)

$$\left(\frac{\partial u}{\partial x}\right)_{t=t_0+\frac{\tau}{2}} = \left(\frac{\partial u}{\partial x}\right)_C + (x - x_0)W + d(y - y_0)$$

$$\left(\frac{\partial u}{\partial y}\right)_{t=t_0+\frac{\tau}{2}} = \left(\frac{\partial u}{\partial y}\right)_C + (y - y_0)V + d(x - x_0)$$

Unknown variables: $|u_R, \left(\frac{\partial u}{\partial x}\right)_C, \left(\frac{\partial u}{\partial y}\right)_C, d$



The pyramidal stencil.

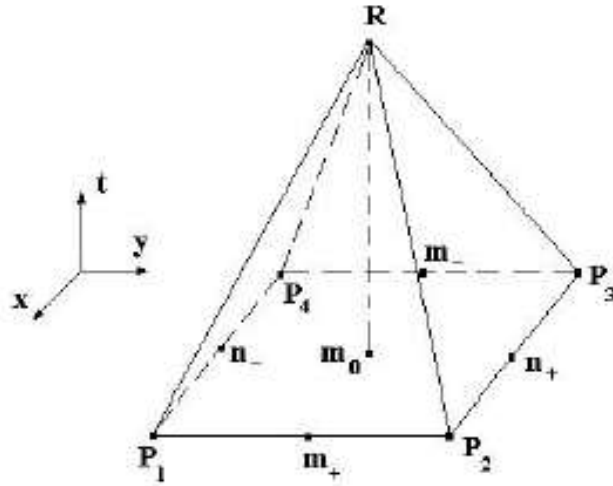
Scheme I

$$W_I = W(m_0) \equiv \frac{1}{3} \left[\frac{u_{P1} - 2u_{n-} + u_{P4}}{h_x^2} + \frac{u_{m+} - 2u_{m_0} + u_{m-}}{h_x^2} + \frac{u_{P2} - 2u_{n+} + u_{P3}}{h_x^2} \right],$$

$$V_I = V(m_0) \equiv \frac{1}{3} \left[\frac{u_{P2} - 2u_{m+} + u_{P1}}{h_y^2} + \frac{u_{n+} - 2u_{m_0} + u_{n-}}{h_y^2} + \frac{u_{P3} - 2u_{m-} + u_{P4}}{h_y^2} \right]$$

Here we take a center point of the middle section of our stencil $(t_0 + \frac{\tau}{2}, x_0, y_0)$ as the point C (or C_I) and choose values W and V (or W_I and V_I) at the middle layer $t_0 + \frac{\tau}{2}$

Numerical Scheme



The pyramidal stencil.

Scheme II

$$\left. \begin{array}{l}
 \text{if } b_1(x_0, y_0, t_0) \geq 0 \text{ and } b_2(x_0, y_0, t_0) \geq 0, \\
 \text{then } W_{II} = W(P_4), \quad V_{II} = V(P_4), \\
 \quad C_{II} = (P_{4x}, P_{4y}, t_0 + \frac{\tau}{2}), \\
 \text{if } b_1(x_0, y_0, t_0) \geq 0 \text{ and } b_2(x_0, y_0, t_0) < 0, \\
 \text{then } W_{II} = W(P_3), \quad V_{II} = V(P_3), \\
 \quad C_{II} = (P_{3x}, P_{3y}, t_0 + \frac{\tau}{2}), \\
 \text{if } b_1(x_0, y_0, t_0) < 0 \text{ and } b_2(x_0, y_0, t_0) < 0, \\
 \text{then } W_{II} = W(P_2), \quad V_{II} = V(P_2), \\
 \quad C_{II} = (P_{2x}, P_{2y}, t_0 + \frac{\tau}{2}), \\
 \text{if } b_1(x_0, y_0, t_0) < 0 \text{ and } b_2(x_0, y_0, t_0) \geq 0, \\
 \text{then } W_{II} = W(P_1), \quad V_{II} = V(P_1), \\
 \quad C_{II} = (P_{1x}, P_{1y}, t_0 + \frac{\tau}{2}).
 \end{array} \right\}$$

Switching criterion - minimal principle

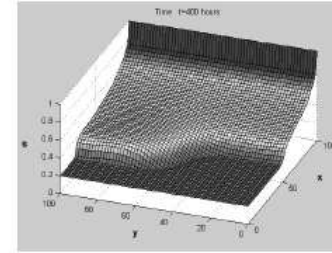
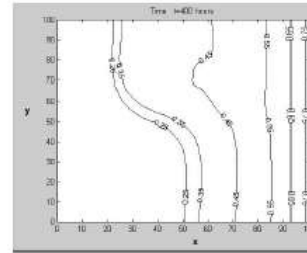
$$u_R^{final} = \min_{i=I,II} \left| u_R^i - \frac{C_0 u_{m_0} + C_1 u_{P_1} + C_2 u_{P_2} + C_3 u_{P_3} + C_4 u_{P_4}}{C_0 + C_1 + C_2 + C_3 + C_4} \right|$$

In results of numerical tests -> the final form

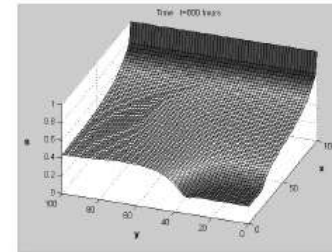
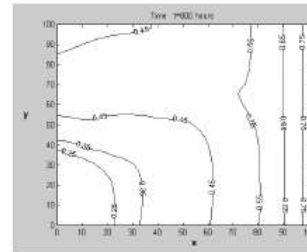
$$u_R^{final} = \min_{i=I,II} |u_R^i - u_{m_0}|$$

Computations

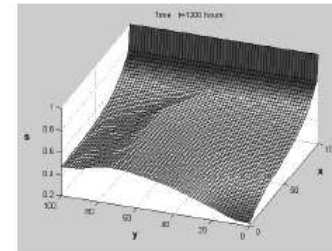
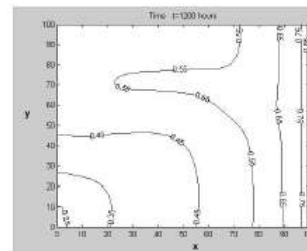
Now let us consider some numerical results obtained by the proposed method. Computations were carried out for the following values of parameters $m = 0.2$, $k_{D_2} = 1.0 \cdot 10^{-12} \text{ m}^2$, $\mu_w = 1 \cdot 10^{-6} \text{ N} \cdot \text{sec} \cdot \text{m}^{-2}$, $\mu_o = 3 \cdot 10^{-6} \text{ N} \cdot \text{sec} \cdot \text{m}^{-2}$, $L = H = 100 \text{ m}$, $P_0 = 0$, $Q_0 = 0.69444 \cdot 10^{-12} \text{ m}^2 \cdot \text{sec}^{-1}$. Parameter k_{D_1} varies in the range from $0.50 \cdot 10^{-12} \text{ m}^2$ to $0.01 \cdot 10^{-12} \text{ m}^2$. Thus the absolute penetration in the subregion D_1 is 2 to 100 times less than those in subregion D_2 . Presented results correspond to the uniform grid with $61 \cdot 61$ nodal points in (x,y) -space.



The water saturation at $t = 400$ hours.
 $k_{D_1} = 0.50 \cdot 10^{-12} \text{ m}^2$.

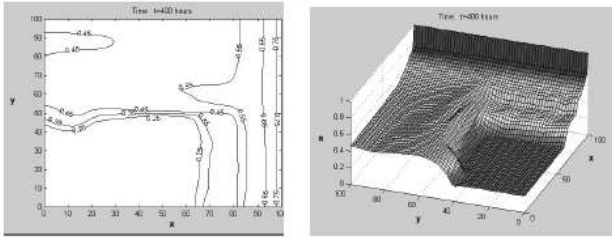


The water saturation at $t = 800$ hours.
 $k_{D_1} = 0.50 \cdot 10^{-12} \text{ m}^2$.

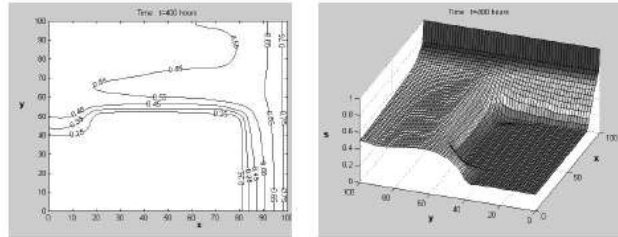


The water saturation at $t = 1200$ hours.
 $k_{D_1} = 0.50 \cdot 10^{-12} \text{ m}^2$.

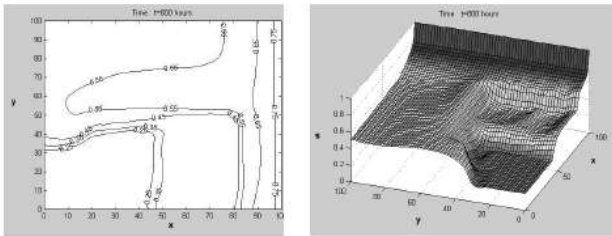
Computations



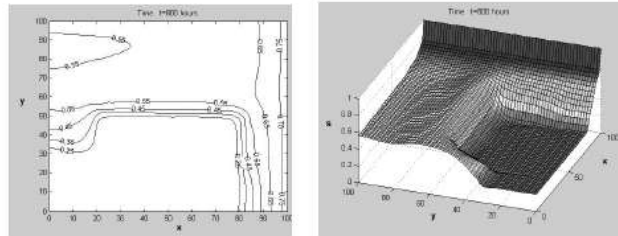
The water saturation at $t = 400$ hours.
 $k_{D1} = 0.20 \cdot 10^{-12} \text{ m}^2$.



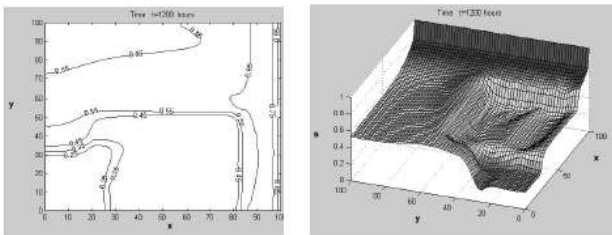
The water saturation at $t = 400$ hours.
 $k_{D1} = 0.01 \cdot 10^{-12} \text{ m}^2$.



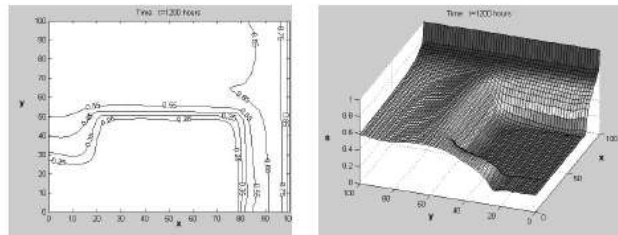
The water saturation at $t = 800$ hours.
 $k_{D1} = 0.20 \cdot 10^{-12} \text{ m}^2$.



The water saturation at $t = 800$ hours.
 $k_{D1} = 0.01 \cdot 10^{-12} \text{ m}^2$.



The water saturation at $t = 1200$ hours.
 $k_{D1} = 0.20 \cdot 10^{-12} \text{ m}^2$.



The water saturation at $t = 1200$ hours.
 $k_{D1} = 0.01 \cdot 10^{-12} \text{ m}^2$.

Computations

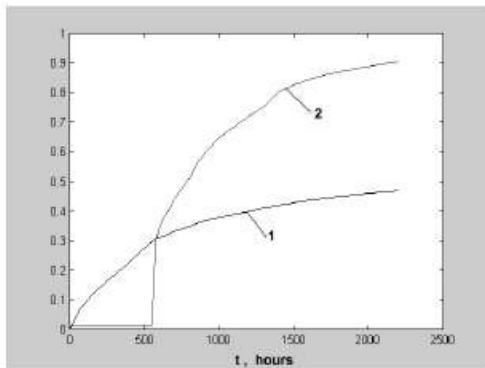
Characteristics of oil recovery efficiency

- ▶ The ratio of the recovery oil to the total oil volume

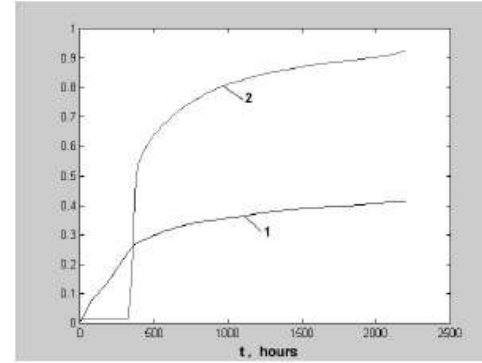
$$\theta(t) = \frac{\int_D [1 - s(x, y, t)] dx dy}{\int_D [1 - s(x, y, 0)] dx dy}$$

- ▶ The water content in the development mixture at the production well

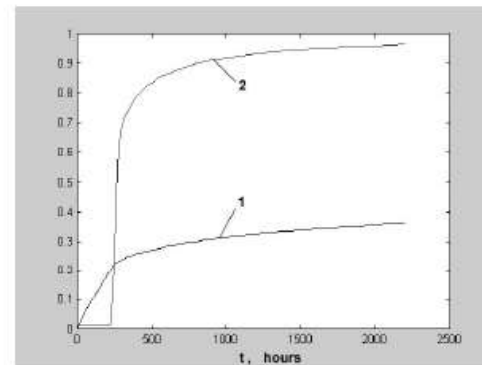
$$\gamma(t) = \frac{\int_0^L [k \frac{k_w}{\mu_w} \frac{\partial p}{\partial x}]_{x=0} dy}{\int_0^L [k (\frac{k_w}{\mu_w} + \frac{k_o}{\mu_o}) \frac{\partial p}{\partial x}]_{x=0} dy}$$



Characteristics of efficiency of oil recovery.
Line 1 - $\theta(t)$, line 2 - $\gamma(t)$. $k_{D1} = 0.50 \cdot 10^{-12} \text{ m}^2$.



Characteristics of efficiency of oil recovery.
Line 1 - $\theta(t)$, line 2 - $\gamma(t)$. $k_{D1} = 0.20 \cdot 10^{-12} \text{ m}^2$.



Characteristics of efficiency of oil recovery.
Line 1 - $\theta(t)$, line 2 - $\gamma(t)$. $k_{D1} = 0.01 \cdot 10^{-12} \text{ m}^2$.

CONCLUSIONS

- ▶ Our high-precision numerical quasi-characteristics technique developed for the transport equation allows us to obtain solutions of complicated porous media problem with essentially heterogeneous parameters without mesh fitting procedures on rough spatial meshes
- ▶ This technique can be implemented even on small computers and workstations for fast evaluation and exact modeling of oil and gas development technological processes

Thanks for attention!